# **Release Note**

Release Date : Aug. 2021 Product Ver. : nGen 2022 (v1.1)

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#### Export the rebar information of nGen to Revit model.

#### Set Rebar Detail Output > Revit Link > Setting > Rebar Detail Structure Define Body Load Boundary Member Analysis Design Results Output Options **E** R R ୍ତ Generate Report Settings Generate Drawing Setting Import Export 🖳 Rebar Detail 🗈 Member Detail Development Length in Tension (Ldt) Rebar Detail 2 ab/Beam/Footing Column/Pedestal Wall/Plate Development Length in Tension (Ldt) General Tension Rebar (unit : m) (Good Development Length in Compression (Ldc) RB9 DB10 DB12 RB6 RB8 Fck Fy Splice Length in Tension (Lst) 0.24 SR24 C30/37 0.12 0.15 0.18 0.20 SD30 C30/37 0.15 0.20 0.22 0.24 0.29 Splice Length in Compression (Lsc) SD40 C30/37 0.20 0.26 0.29 0.33 0.39 Development Length of Std. Hook in Tension (Ldh) SD50 C30/37 0.24 0.33 0.36 0.41 0.48 Auto-calculation(Batch) < Consider Top Tension rebar detail. (Poor) Tension Rebar placed top over fresh concrete (unit : m) RRA RB8 RB9 **DB10 DB12** Fy Fck SR24 C30/37 0.17 0.23 0.25 0.28 0.34 C30/37 SD30 0.21 0.28 0.32 0.35 0.42 C30/37 0.28 0.37 0.42 0.46 0.55 SD40 C30/37 0.35 0.47 0.52 0.58 0.70 SD50 Auto Calculator EN1992-1-1-2004 4 Reference Code 5 6

Set the length for the development and splice of the rebar. Auto-calculation only supports Eurocode. For other design codes, copy & paste the values calculated in Excel.

- Click the type of development or splice.
- Select the member type tap.
- Click "Auto Calculator".
- Select the reference code.
- 6 Click "V" button.
- G Click "V" button.

### Export the rebar information of nGen to Revit model.



### Export the rebar information of nGen to Revit model.

## Member detail & Default setting

Member detail for rebar shape X	Member detail for rebar shape X	Member detail for rebar shape X	Member detail for rebar shape X
Beam  V Selected 1 Object(s)	Column  V Selected 1 Object(s)	Wall Selected 1 Object(s)	Slab  V Selected 1 Object(6)
Start/Top Start/Top(in) End/Top Start/Bigttom Start/Bigttom Type 1 Type 2 Type 3 Type 4 Type 5 Type 6	Type 1 Type 2 Type 3 Type 4 Type 5 Type 8	Storr, Type 1 Type 2 Type 3 Type 4 Type 5 Type 6	X-Etart/Top X-Etart/Bot X-Start/Bot X-Start/Bot Typo 1 Typo 2 Typo 3 Typo 4
Rebar Shape	Rebar Shape	Rebar Shape	Rebar Shape
Start/Top         Type 2         End/Top         Type 2            Start/Bottom         Type 1         End/Bottom         Type 1            Stirrup         Type 5	Dowel bar of footing         Top         Type 1         Bottom         Type 3            Dowel 1         Type 1         V         Dowel 2         Type 1            Hoop         Type 6         V	Dowel bar of footing           Ver./Top         Type 1         Ver./Bottom           Dowel 1         Type 1         Dowel 2         Type 2           Hor./Start         Type 1         Hor./End         Type 1	X-Start/Top         Type 2         X-End/Top         Type 2            X-Start/Bot.         Type 1          X-End/Bot.         Type 1            Y-Start/Top         Type 2         Y-End/Top         Type 1
Length Method	Length Method	End hoop Type 5 V	Y-Start/Bot. Type 1 V-End/Bot. Type 1 V
Start/Top         Ldh         End/Top         Ldh            Start/Bottom         Ldc         End/Bottom         Ldc            Start/Top(In)         0.30         L         End/Top(In)         0.30         L           Start/Bottom         L         End/Top(In)         0.30         L         End/Top(In)         0.30         L	Top     Lst     V     Bottom       Dowel 1     Lst     V     Dowel 2     Ldt	Length Method     ^       Ver./Top     Lst     Ver./Bottom       Dowel 1     Ldt     Dowel 2       Hor./Start     Ldt     Hor./End	Length Method         ∧           X-Start/Top         Ldh         ∨         X-End/Top         Ldh         ∨           X-Start/Bot         Ldc         ∨         X-End/Bot         Ldc         ∨           Y-Start/Top         Ldh         ∨         Y-End/Top         Ldc         ∨           Y-Start/Top         Ldh         ∨         Y-End/Top         Ldn         ∨
Beam Type	Column Type	Shear Wall	Slab Type

### Export the rebar information of nGen to Revit model.

## Member detail & Default setting



#### Export the rebar information of nGen to Revit model.

## Export nGen model to Revit



#### Export the rebar information of nGen to Revit model.

#### Scope of support

Items (Member)	Supporting	Detail
Beam / sub Beam / Footing Girder	0	
Column / sub Column / Pedestal	0	• Support to input rebar in critical zone of beam, column and wall.
Wall / Basement Wall	0	• Support to input dowel rebar for wall and column.
Plate	0	• Support the rebar data to 2D members with slopes and irregular s
Slab / Flat Slab	0	hapes
Isolated / Strip / Mat Footing	0	
Brace	X	
Wall column / Plate Beam	X	

• Development type : Support the straight and 90° hook shape (135° hook in hoop bar)

• Hook dimension : Use the auto-calculation in Revit

Rebar Hook Lengths									
Rebar Bar Type:	Ri	ebar Bar Diameter:							
P12	:	12.0 mm							
Rebar Hook Length can be automatically calculated based on the Rebar Hook Extension Multiplier property, or the Hook Len can be manually overridden here. The Offset Length is optional and is only used for scheduling									
Rebar Hook Length can be automatically ca can be manually overridden here. The Off	aculated based on the Rel set Length is optional and	is only used for schedu	ling						
Rebar Hook Length can be automatically ca can be manually overridden here. The Offi Rebar Hook Type	Aculated based on the Rel set Length is optional and Auto Calculation	Hook Extension Multi	Tangent Length	O'					
Rebar Hook Length can be automatically ca can be manually overridden here. The Offi Rebar Hook Type ∑Standard - 90 deg. ∑Standard - 180 deg.	Auto Calculation	Hook Extension Mult is only used for schedu Hook Length 196.0 mm	Tangent Length 196.0 mm 100.0 mm	O 104.0					
Rebar Hook Length can be automatically ca can be manually overridden here. The Off Rebar Hook Type Standard - 90 deg. Standard - 180 deg. Stirrup/Tie - 90 deg.	Auto Calculation	Hook Extension Mult is only used for schedu Hook Length 196.0 mm 140.5 mm 109.0 mm	Tangent Length 196.0 mm 100.0 mm 109.0 mm	к Length О <sup>.</sup> 104.0					
Rebar Hook Length can be automatically ca can be manually overridden here. The Off Rebar Hook Type Standard - 90 deg. Standard - 180 deg. Stirrup/Tie - 90 deg. Stirrup/Tie - 90 deg.	Auto Calculation	Hook Extension Multi is only used for schedu Hook Length 196.0 mm 140.5 mm 109.0 mm 108.0 mm	Tangent Length 196.0 mm 100.0 mm 109.0 mm 109.0 mm	or 104.0 70.2 r					



#### **Add National Annex**

Design §	Settings								•	×
General	Steel	RC	Rebar	Checking C	ontrol					
Des	sign Co	ode								
Cod	e Categ	jory						EuroCo	ode 🗸	
Nati	onal An	nex					Unite	d Kingd	om 🗸	
Stee	el						Reci	owwe	nded	
HL Ru	T	a of C	*****					Mala	aysia	
_ = 3₩ ⊻ di	ay iyp roction	e 01 5	tructur					Ciner		
Vdi	rection						Unite	Singa	apore	
= Say	vina Tv	ne of	Desian				Onite		hnele	
Sav	ing Tvp	e of De:	sign Re	sult		for Cri	itical Con	dition U	nivivi	
	• •		·							
Natio	nal Anr	nex								
Select	the nati	onal an	nex,							
		_	_	r						
Reset		eset by F	Program					$\sim$	+	Х

## Applied UK & Singapore & Ireland NA for EN1993-1-1:2005

Prevision	RECOMMENDED	UK (United Kingdom) & Singapore	Ireland
<b>6.1(1)</b> partial safet y factor (default)	γ <sub>M2</sub> = 1.25	γ <sub>M2</sub> = 1.10	γ <sub>M2</sub> = 1.25
<b>6.3.2.3(1)</b> parameter λ <sup>LT,0</sup>	$\lambda_{LT,0} = 0.4$	$\lambda_{LT,0} = 0.4$ (rolled) $\lambda_{LT,0} = 0.2$ (welded)	$\lambda_{LT,0} = 0.4$
<b>7.2.1(1)B</b> deflection li mit (default)	Beam = L/250 Colum =L/250	Beam = L/360 Colum = L/300	Beam = L/360 Colum = L/300
<b>Annex B.</b> kyy~kzz	calculated as Annex A.	calculated as Annex A (UK). max( $\lambda$ z, $\lambda$ LT) (Singa pre)	calculated as Annex B.
[ EN 1993-1 -5 ] 5.1(2) NOT E 2	Fy ≤ 460 : 1.2 ETC : 1.0	All : 1.0 (Only UK)	All : 1.0

## Applied UK & Singapore & Ireland NA for EN1992-1-1:2004

Prevision	RECOMMENDED	UK(United Kingdom) & Singapore	Ireland
<b>3.1.2(2)</b> value of C <sub>max</sub>	C90/105	() Alternatively, <u>shear strength of concr</u> ete strength classes higher than C50/60 may be limited to that of C50/60	() Alternatively, <u>shear strength of concr</u> <u>ete strength classes higher than C50/60</u> <u>may be limited to that of C50/60</u>
<b>3.1.6(1)</b> value of $\alpha_{cc}$ (default)	$\alpha_{cc} = 1.0$	$\alpha_{cc} = 0.85$	$\alpha_{cc} = 0.85$
$\begin{array}{c} \textbf{6.2.2(1)} \\ \text{value of } C_{\text{Rd,c}},  v_{\text{min,}} \\ \rightarrow V_{\text{Rd,c}},  \sigma_{\text{cp,limit}} \end{array}$		use the reduced fck $\rightarrow$ refer 3.1.2(2)	use the reduced fck $\rightarrow$ refer 3.1.2(2)
<b>6.2.2(6)</b> value of v		use the reduced fck $\rightarrow$ refer 3.1.2(2)	use the reduced fck $\rightarrow$ refer 3.1.2(2)
<b>6.2.3(3)</b> value of v <sub>1</sub> and $\alpha_{cw}$	v <sub>1</sub> = 0.6 (f <sub>ck</sub> <60MP) v <sub>1</sub> = 0.9-f <sub>ck</sub> /200 > 0.5 (f <sub>ck</sub> > 60MPa)	[ If the design stress of the shear reinforc ement is below 80% of the characteristic yield stress fyk, ] $v_1 = 0.54(1-0.5*\cos\alpha) (f_{ck}<60MP)$ $v_1 = (0.84-f_{ck}/200)*(1-0.5*\cos\alpha) > 0.5$ $(f_{ck} > 60MPa)$	[ If the design stress of the shear reinforc ement is below 80% of the characteristic yield stress fyk, ] $v_1 = 0.54(1-0.5^*\cos\alpha) (f_{ck} < 60MP)$ $v_1 = (0.84-f_{ck}/200)^*(1-0.5^*\cos\alpha) > 0.5$ $(f_{ck} > 60MPa)$
<b>6.4.5(3)</b> v <sub>Ed</sub> limit	v <sub>Rd,max</sub> = 0.5vf <sub>cd</sub>	$v_{Rd,max} = 0.5vf_{cd}$ In addition, $v_{Ed}$ should be limited to $2v_{Rd,c}$ at the first control perimeter.	$v_{Rd,max} = 0.5vf_{cd}$ In addition, $v_{Ed}$ should be limited to $2v_{Rd,c}$ at the first control perimeter.
9.5.3(3) column transverse reinforcement	s <sub>cl,tmax</sub> = min[20D <sub>bar,min</sub> , Hc, Bc, 400mm]	$s_{cl,tmax}$ = min[20D <sub>bar,min</sub> , Hc, Bc, 400mm] check α <sub>n</sub> α <sub>s</sub> ω <sub>wd</sub> ≥0.04 (>C50/60)	$s_{cl,tmax}$ = min[20D <sub>bar,min</sub> , Hc, Bc, 400mm] check α <sub>n</sub> α <sub>s</sub> ω <sub>wd</sub> ≥0.04 (>C50/60)



### Applied UK & Singapore & Ireland NA for EN1992-1-1:2004

Prevision	RECOMMENDED	UK(United Kingdom) & Singapore	Ireland
Category H : roofs	$\psi 0 = 0.0$ $\psi 1 = 0.0$ $\psi 2 = 0.0$	$\psi 0 = 0.7$ $\psi 1 = 0.0$ $\psi 2 = 0.0$	$\psi 0 = 0.6$ $\psi 1 = 0.5$ $\psi 2 = 0.0$
A1.2.2 (Table A.1.1) Wind	ψ0 = 0.6 ψ1 = 0.2 ψ2 = 0.0	ψ0 = 0.5 ψ1 = 0.2 ψ2 = 0.0	ψ0 = 0.6 ψ1 = 0.2 ψ2 = 0.0

Auto Generation Options								×
		EN	1992-1-1-2004					
Options								
General Ortho. Group Response Spectrum	Factor for	Variable A Code Base	ction	Singapore			~	
Loop Seismic		oad Set	Load Type	Category	psi0	psi 1	psi2	
Factor for Variable Action		Wind	Wind Load o	Wind	0.50	0.20	0.00	
		Snow	Snow Load	Snow	0.00	0.00	0.00	
		Temp	Temperature	Temperature	0.60	0.50	0.00	

Factor for Variable Action in Load Combinations

## Add Singapore NA for Wind Load (EN1991:2005) & Seismic Load (EN1998:2004)

Design Wind	Load (E	Building)		×			
Name	EN 1991	L:2005(Bui	lding)-1				
Code	EN1991:2005 ~						
National Annex	Singapo	ore		~			
Average Roof H	eight (H)	)		30  m			
Common Para	ameters			^			
Structure Type	•	2	~	7			
Fund Basic Wir	nd Veloci	ty	20	m/s			
Directional Fac	tor		1				
Seasonal Facto	or		1	<u>i</u>			
Turbulence Fa	ctor		1	Ī			
Orographic Ef	fects			~			
Consider E	ffects						
Orography Cat	egory	Cliffs or E	Escarpme	nt 🗠			
Building Locatio	n	Upwind		~			
Height of Topo	graphic f	eature		5 m			
Length of Upwi	nd Slope			10 m			
Length of Down	nwind Sk	ope		0 m			
Crest Building D	Distance			10 m			
Effects of Nei	ghbouri	ing High-	rise Stru	ctures ^			
Consider E	ffects						
Avg. Height of	Nearby	Structure	s	25 m			
Distance to the	e High-Ri	ise		45 m			
Feature of th	ne Neigh	bouring Hi	gh-rise S	tructure			
Larger Horiz	ontal Din	nension		30 m			
Height				50 m			
Raising of Dis	placem	ent Heigh	t	^			
Consider E	ffects						
Obstruction He	eight			0 m			
Upwind Spacin	g			0 m			
Structural Fac	tor			^			
Structural Fact	tor		1				
Report			$\checkmark$	$+ \times$			

Design Seismic Load									
Name	Name EN 1998: 2004(Static)-1								
Design EN1998:2004									
National Annex	Singapor	e				~			
Seismic Load	Paramete	ers				^	1		
Ground Type		С		_	~		1		
Response Spec	ctrum Typ	e H	orizo	ntal E	ia 🗸				
C Spectrum Par	rameters								
O Automatio	:	$\odot$	User	Defin	ed				
Soil Factor	r <b>(</b> S)	1.6	ть		0.4				
	Tc	1.1	Тd		10.4				
Ref. Peak Grou	und Acc.			C	.0178	g			
Behavior Facto	r			1.5					
Lower Bound F	actor				0.2				
Importance Fa	ctor		1.4	4	~				
Viscous Dampir	ng Ratio				5	%			
Structural Day	amatarc					~	1		
Structural Par	ameters								
		Ma	jor	Ort	ho				
Analytical F	Period	0		0					
Approxima	te Period	1		2					
Fundamental P	eriod	1		2		sec			
Report			~	/	+	)	×		

Dialog Box for Static seismic Load

	Design Spect	rum						×
	Name	EN 1998	2004	1(R	S)-1			
	Design	EN 1998:	2004	1			~	
	National Annex	Singapo	re				~	
	Seismic Load	Paramet	ers				~	]
	Ground Type			с		~		
	Response Spe	ctrum Typ	e e	Но	orizonta	Ela 🗸		
	Spectrum Pa	rameters		_				
	O Automatio	:		<u>)</u> u	Jser Def	fined		
	Soil Facto	r(S)	1.	6	ть	0.4		
		Тс	1.	1	Td	10.4		
	Ref. Peak Grou	und Acc.				0.0178	g	
	Behavior Facto	or				1.5		
	Lower Bound F		0.2					
	Importance Fa	1.4	~	]				
	Viscous Dampir	ng Ratio				5	%	
R	) Report				$\checkmark$	+	>	~
					×		- '	Ì

**Dialog Box for Response Spectrum** 

## 3. Thailand Code : DPT (Wind and Seismic load)

## Add DPT.1311-50:2007 (Wind Load) & DPT,1301/1302-61:2018 (Seismic Load)

		Design Wind Load (Building) $ imes$	Design Seismic Load $ imes$	Design Spectrum			
		Name DPT. 1301/1311(Building)-1	Name DPT.1301/1311(Static)-1	Name DPT. 1301/1311(RS)-1			
	มยพ. <b>1311-50</b>	Code DPT.1311-50:2007 ~	Design DPT.1301/1302-61:2018 ~	Design DPT. 1301/1302-61:2018 ~			
		National Annex None ~	National Annex None ~	National Annex None ~			
มาตรฐา	านการด้านวณแรงสม	Average Roof Height (H) 9 m	Coismis Load Decemeters	Soirmir Load Darameterr			
และการต	ตอบสนองของอาดาร	Application Method	Region				
		Method	O Bangkok Region except Bangkok	O Bangkok Region except Bangkok			
		O Simplified Detailed	Method	Method			
			By Graph 1.4.6~7 O By Table 1.4-4~5	O By Graph 1,4.6~7 O By Table 1.4-4~5			
		Common Parameters	Seismic Zone	Seismic Zone			
		Wind Zone Zone 1 V					
a militarad	J8W.1301/1302-61	Basic Wind Speed 25 m/sec	Site Class	Site Class			
		Terrain Category B					
	มาตรฐานการออกแบบอาดารตานทาน		Ss 0.75 Pa 1.200 Sds 0.000 g	Ss 0.75 V Fa 1200 Sds 0.000 g			
- 1. M	การสันสะเทือนของแพ่นดินไหว	Topographic Effects	S1 0.3 V PV 1.000 Sd1 0.300 g	SI 0.3 PV 1.800 Sai 0.300 g			
		Include Topographic Effects	Period Coef. (Cu) 1.500	Category			
and the second s		Hill Shape 2-D Ridge or Val Y	Category				
		Building Location Downwind					
		Hill Height 0 m		Structural Parameters			
(1988)		Hill Length 0 m	Seismic Design Category	Response Modification			
		Crest-Building Distance 0 m		Factor (R)			
		Gust Factor and Pressure Coefficient	Structural Parameters				
		Auto Calculate by Structure Information	Major Ortho	Dialog Box for Response Spectrum			
		Gust Factor (Cg)	Analytical Period 0 0				
		Major 2.5 Ortho. 2.5	O Approximate Period 0				
			Fundamental Period 0 0 sec				
		Additional Parameters ^	Response Modification 4 4 4				
	η, τ <del>ά</del> ,	Cross Wind	Damping 0.025 V				
	กรมโยธาธิการแล:พังเมือง	Response ( Disp. / Accel, )					
	กระทรวงมหาดไทย	Response (Disp. / Accel. )	Dialog Box for Static seismic Load				
	W. <b>dl</b> . 2561						
		Dialog Box for Wind Load					

## 4. Thailand DB : Rebar & Material

#### Add Rebar DB and material as per TIS

Design Settings											
General Steel	RC Rebar	Checking Control									
Material       Rebar Material         Rebar Arrangement       Material Standards       TIS          Alternate Rebar       Setting range and s       Batch Setting          Main Rebar Grade       SR24         Shear Rebar Grade       SD30         SD50											
Set Rebar material											
Tensile Strength Yield Strength Fu (Mpa) Fy (Mpa)											
SR 24		385 235									
SD 30		480	295								
SD 40		560	390								
SD 50		620	490								

Beam	Column	Brace	Plate Wall Slab / Foundat		Slab / Foundatio	on B	asement Wall			
	Diamete	Main Rebar					Shear Rebar			
	r	Use Fy				Use	:	Fy		
	RB6		SR24		235000000.00	<ul> <li>Image: A start of the start of</li></ul>	SR24	235000000.0		
	RB8		SR24		235000000.00	<b>~</b>	SR24	235000000.0		
	RB9		SR24		235000000.00	<b>~</b>	SR24	235000000.0		
	DB10		SD40		39000000.00	<b>~</b>	SD40	39000000.0		
	DB12	~	SD40		39000000.00	~	SD40	39000000.0		
	DB16	<ul> <li>Image: A set of the set of the</li></ul>	SD40		39000000.00		SD40	39000000.0		
	DB20	<ul> <li>Image: A set of the set of the</li></ul>	SD40		39000000.00		SD40	39000000.0		
	DB22	✓	SD40		39000000.00		SD40	39000000.0		
	DB25	✓	SD40		39000000.00		SD40	39000000.0		
	DB28	<ul> <li>Image: A set of the set of the</li></ul>	SD40		39000000.00		SD40	39000000.0		
	DB32	<ul> <li>Image: A start of the start of</li></ul>	SD40		39000000.00		SD40	39000000.0		
	DB36		SD40		39000000.00		SD40	390000000.0		

#### Rebar strength as per TIS

#### Rebar DB as per TIS & Setting range and strength by diameter

EN1992-1-1-2004(Recommended)															
Beam	Column E	ace	Plate	Wall	Slab	Flat Slab	Footing Girder	Pedestal	Basement Wall	Isolated Footing	Strip Foot	ing Mat Footi	ng		
	Section Donth		Main Rebar					Shear Rebar					Skin Bar		
- 4	Secuon Depun	Min. Dia	meter	Max.	Diameter	Max. Lay.	Min. Diar	neter Max.	Diameter Min.	Spacing	Increment	Max. Spacing	Min. Diameter	Max. Dia	
	0.30 DB12		DB16		1 Layer	RB6	DB12		0.10	0.05	0.15	DB12	DB16		
	(	.35	DB12		DB16		1 Layer	RB6	DB12		0.10	0.05	0.15	DB12	DB16
	(	.40	DB12		DB20		1 Layer	RB6	DB12		0.10	0.05	0.20	DB12	DB16
	(	.50	DB16		DB20		1 Layer	RB6	DB12		0.10	0.05	0.25	DB12	DB16
	(	.60	DB16		DB25		1 Layer	RB6	DB12		0.10	0.05	0.25	DB12	DB16
	(	.70	DB16		DB25		1 Layer	RB6	DB12		0.10	0.05	0.25	DB12	DB16
	(	0.80	DB16		DB25		1 Layer	RB6	DB12	_	0.10	0.05	0.25	DB12	DB16

## 5. Improvement of Snap

#### Separate Cad snap and Modeling Snap

- Improved modeling convenience by separating cad snap and model snap
- Control the default settings for cad snap in Global settings.

